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Closure to “improving prediction of dam failure peak outflow using neuroevolution combined with k-Means clustering” by Amir Hossein Eghbali, Kourosh Behzadian, Farhad Hooshyaripor, Raziye Farmani, and Andrew P. Duncan

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1 **Closure to Discussion of “Improving Prediction of Dam Failure Peak Outflow Using**
2 **Neuroevolution Combined with K-Means Clustering” by Amir Hossein Eghbali, Kourosh**

3 **Behzadian, Farhad Hooshyaripor, Raziye Farmani, and Andrew P. Duncan,**

4 **July 2017, Vol. 22, No. 6.**

5 **DOI: 10.1061/(ASCE)HE.1943-5584.0001505**

6 **By Farhad Hooshyaripor¹ and Kourosh Behzadian²**

7
8 The authors would like to first thank the discusser for making three constructive comments which
9 can be divided into two groups: (1) comments 1 and 2 related to clarity of a function used in the
10 model developed and accuracy of the data collected; (2) comment 3 related to the ability of the
11 developed model to predict new peak discharges of dam failure. To close the discussion and further
12 clarification, the following are noted for each of the comments:

13
14 **Comment #1:**

15 The authors of the original paper (Eghbali et al. 2017) applied MATLAB tool as a platform to
16 generate and combine artificial neural network (ANN) with genetic algorithm (GA) and *k*-means
17 clustering. However, due to the combination of ANN and GA, the standard ANN as noted by the
18 discusser was not used and instead all steps were coded in MATLAB. Thus, the tangent sigmoid
19 (*tansig*) transfer function used in the paper for generating ANN was not the default MATLAB

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20 function denoted as *tansig*(n) in MATLAB. Instead, The following *tansig* transfer function was
21 used in the paper (Araghinejad 2014):

$$22 \quad \textit{tansig}(x) = 2/(1+e^{-ax})-1=(1-e^{-ax})/(1+e^{-ax}) \quad a>0 \quad (1)$$

23 where a = constant parameter which was considered 1.

24 The authors did not carry out sensitivity analysis for parameter a but even if the default MATLAB
25 function is used instead (i.e. $a=2$), it is unlikely to lead to less accurate predictions although
26 different weights and biases may be obtained for the same database.

27

28 **Comment #2:**

29 The discussor highlighted the importance of the discrepancy of peak flow rates from the failure of
30 three dams (i.e. Oros Dam in Brazil, Banqiao Dam in China and Hell Hole Dam in the United States)
31 reported by the original paper and other publications. As the accurate date is of paramount
32 importance to the results, the authors endeavoured to collect the data from different sources and
33 impartially pick up those that were widely used. More specifically, failure peak flow rate of Oros
34 Dam used in the paper (i.e. 9,630 m³/s) has been reported by several sources (e.g. Wahl 1998; Xu
35 and Zang 2009; Pierce et al. 2010; and Thornton et al. 2011) while the value expressed by the
36 discussor (i.e. 58,000 m³/s which is around 6 times larger) can be found in only few works (e.g.
37 Wahl 2014). In addition, the peak flow rate of 56,300 m³/s for the Banqiao Dam failure was only
38 used by the discussor's publication while peak flow rate of 78,000 m³/s was reported by many
39 independent researchers (e.g. Fujia and Yumei 1994; Xu and Zhang 2009; Pierce et al.
40 2010; Thornton et al. 2011). Due to large amount of peak discharge in these two data samples (i.e.
41 Banqiao and Oros Dams), we also agree that the major difference in the collected data can directly

42 affect any developed model. For example, Banqiao Dam failure is an important data sample due
43 to having the highest peak discharge in the database. Also, the original paper used the widely-
44 reported value of 7,360 m³/s for Hell Hole dam failure (MacDonald and Langridge-Monopolis
45 1984; Wahl 1998; Xu and Zhang 2009) while peak discharge of 17,000 m³/s has only been used
46 by the discussor. In addition, the frequency of the 92 data samples analysed in the paper shows
47 that most of the observed peak flow discharges are less than 10,000 m³/s (Hoosyaripor et al. 2014).
48 In this dataset, there are only one peak discharge over 70,000 m³/s, 2 cases over 60,000 m³/s, 3
49 cases over 30,000 m³/s and 7 cases over 20,000 m³/s among 92 data samples.

50 It should also be noted that there is only one predictive model which is trained for all clusters using
51 90% of all data samples, not based on the data samples in one cluster (e.g. Oros and Banqiao
52 Dams) as noted by the discussor. In addition, due to insufficient data available (92 samples),
53 conventional model verification (i.e. dividing database into two subsets of training and test) was
54 inefficient. Hence, the cross-validation technique was used in the paper, implying that all 92 data
55 samples were participated in the evaluation of the test set (see “Assessment of Performance
56 Indicators” section in the original paper).

57

58 **Comment #3:**

59 The comment challenges overfitting of the clustered ANN-GA model. As noted by the discussor,
60 overfitting often occurs when the number of hidden neurons is large (i.e. model is excessively
61 complex) while the proposed model has only four neurons which is far less than the number of
62 data pairs (i.e. 82-83 equal to 90% of data samples). In other words, if the number of parameters
63 in the ANN is much smaller than the total number of points in the training set which is the case in

64 the paper, there is little chance of overfitting (MATLAB). Also, to avoid overfitting, conventional
65 ANNs divide the database into two subsets of training and validation, in which the training dataset
66 will only participate in the model training. Then, the ANN training will carry on to improve the
67 fitness on training dataset until the mode performance on validation dataset (i.e. independent and
68 unseen date) is deteriorating. Similarly, the cross-validation technique was used in the proposed
69 model as unseen data to avoid overfitting during the model training (Eghbali et al. 2017).

70 Furthermore, the authors totally agree with the trends of the profile traces shown in the discussion
71 for the model developed. However, the reason for the unexpected functional responses in some
72 profile traces cannot be attributed to a flaw or overfitting in the developed model but it is related
73 to the discrepancy of the collected data. More specifically, cluster #1 has only two members (i.e.
74 Oros and Banqiao Dams) in which the profile trace for constant value of H_w is the opposite of the
75 expected function response (Fig. 1a in the discussion paper). When looking at the data of these
76 two dams, it is apparent that given relatively similar H_w around 33m for both dams and a large
77 water volume above the breached invert (V_w) in Oros Dam (660 mcm) compared to Banqiao Dam
78 (607.5 mcm), the peak flow discharges are considerably opposite (78,100 m³/s for Banqiao Dam
79 compared to 9,630 m³/s for Oros Dam). The other unexpected functional response is related to
80 variation of Q_p with V_w in cluster #3 (Fig. 1c in the discussion paper) which has 3 members.
81 Similar discrepancy can be observed within the dataset of these members. More specifically,
82 coefficient of determination (R^2) between V_w and Q_p is significantly low (i.e. $R^2=0.21$) (Eghbali et
83 al. 2017). Interestingly, the correlation between H_w and Q_p in the members of the same cluster is
84 very strong (i.e. $R^2=0.98$) which also confirms the profile trace shown in Fig. 2c in the discussion
85 paper. Similar correlation is in place for the last expected functional response (i.e. Fig. 1d of the
86 discussion paper related to variation of Q_p with V_w in cluster #4. Although this cluster has

87 relatively large number of data samples (i.e. 18 members), a weak correlation is observed between
88 the members (i.e. $R^2=0.02$).

89 As can be seen in the above discussion, most of the inaccuracies and unexpected functional
90 responses are mainly referred to the discrepancies between the data collected. Although the highly
91 controversial data (e.g. the data in clusters #1 and #3 and some of the uncorrelated/outlier data in
92 cluster #4) can be simply removed and the problem can be apparently solved, the authors do not
93 recommend it due to the limited number of data available. Instead, the key message of the original
94 paper is to identify the similar attributes of the data and conduct data clustering to recognize
95 different specifications and predict their peak failure flows more accurately than the previously
96 developed models including conventional regression models.

97 In addition, it seems to be inappropriate to stand by for future dam failures to enrich the quality
98 and quantity of the database of dam failure as it will be unlikely to observe these catastrophic
99 phenomena frequently in the future due to the advance in monitoring systems. Therefore, although
100 some data collected from various breach cases may seem to be statistically chaotic, every piece of
101 data may reveal information of high relevance (Gupta and Singh 2012) and hence they should not
102 be changed/removed in favour of achieving a better correlation of the developed model for dam
103 failure analyses.

104 **References**

105 Araghinejad, S. (2014). Data-Driven Modeling: Using MATLAB: registered: in Water Resources
106 and Environmental Engineering. Water Science and Technology Library, vol(67), page
107 146.

108 Eghbali, A. H., Behzadian, K., Hooshyaripor, F., Farmani, R., & Duncan, A. P. (2017). Improving
109 prediction of dam failure peak outflow using neuroevolution combined with k-means
110 clustering. *Journal of Hydrologic Engineering*, 22(6), 04017007.

111 Fujia, T., and Yumei, L. (1994). "Reconstruction of Banqiao and Shimantan dams." *Int. J.*
112 *Hydropow. Dams*, 49–53.

113 Gupta, S. K. and Singh V. P. (2012). Discussion of "Enhanced Predictions for Peak Outflow from
114 Breached Embankment Dams" by Christopher I. Thornton, Michael W. Pierce, and Steven
115 R. Abt, *J. Hydrol. Eng.*, DOI: 10.1061/(ASCE)HE.1943-5584.0000288.

116 Hooshyaripor, F., Tahershamsi, A., and Golian, S. (2014). "Application of copula method and
117 neural networks for predicting peak outflow from breached embankments." *J. Hydro-*
118 *Environ. Res.*, 8(3), 292–303.

119 MATLAB [Computer software]. MathWorks, Natick, MA

120 MacDonald, T. C., and Langridge-Monopolis, J. (1984). "Breaching characteristics of dam
121 failures." *J. Hydraul. Eng.*, 1105, 567–586.

122 Pierce, M. W., Thornton, C. I., and Abt, S. R. (2010). "Predicting peak outflow from breached
123 embankment dams." *J. Hydrol. Eng.*, 15(5), 338–349.

124 Thornton, C. I., Pierce, M. W., and Abt, S. R. (2011). "Enhanced predictions for peak outflow
125 from breached embankment dams." *J. Hydrol. Eng.*, 10.1061/(ASCE)HE.1943-
126 5584.0000288., 81–88.

127 Wahl, T. L. (1998). "Prediction of embankment dam breach parameters—A literature review and
128 needs assessment." U.S. Bureau of Reclamation Dam Safety Rep. No. DSO-98-004,
129 Denver.

130 Wahl, T. L. (2014). "Evaluation of erodibility-based embankment 144 dam breach equations." 145
131 *Hydraulic Laboratory Report HL-2014-02*, U.S. Department of the Interior, Bureau of 146
132 Reclamation, Denver, Colorado

133 Xu, Y., and Zhang, L. M. (2009). "Breaching parameters for earth and rockfill dams." *J. Geotech.*
134 *Geoenviron. Eng.*, 10.1061/(ASCE)GT .1943-5606.0000162, 1957–1970.